

Centre Scientifique et Technique du Bâtiment

84 avenue Jean Jaurès CHAMPS-SUR-MARNE F-77447 Marne-la-Vallée Cedex 2

Tél.: (33) 01 64 68 82 82 Fax: (33) 01 60 05 70 37





European Technical Assessment

ETA-12/0125 of 04/06/2018

English translation prepared by CSTB - Original version in French language

General Part

Nom commercial Trade name

Famille de produit Product family

Titulaire *Manufacturer*

Usine de fabrication Manufacturing plant

Cette évaluation contient: This assessment contains

Base de l'ETE Basis of ETA

Cette Evaluation remplace : This Assessment replaces :

Injection system CV.VSF PRO

Cheville à scellement de type "à injection" pour fixation dans le béton : M8 à M24, fers à béton 8 à 25mm

Bonded injection type anchor for use in concrete: sizes M8 to M24, rebar 8 to 25mm

Vorpa SRL Via San Leo 5 47036 RICCIONE (RN)

Italy

Italy Plant 1

22 pages incluant 19 pages d'annexes qui font partie

intégrante de cette évaluation

22 pages including 19 pages of annexes which form an

integral part of this assessment

DEE 15-33-0499-06.01 EAD 15-33-0499-06.01

ETA-12/0125 du 13/05/2013 ATE-12/0125 dated 13/05/2013

Translations of this European Technical Assessment in other languages shall fully correspond to the original issued document and should be identified as such. Communication of this European Technical Assessment, including transmission by electronic means, shall be in full. However, partial reproduction may be made, with the written consent of the issuing Technical Assessment Body. Any partial reproduction has to be identified as such.

Specific part

1 Technical description of the product

The injection system CV.VSF PRO is a bonded anchor (injection type) consisting of a mortar cartridge with Vorpa resin CV.VSF PRO and a steel element. The steel elements are threaded rods made of zinc coated steel, stainless steel, high corrosion resistant stainless steel (HCR), or rebar.

The steel element is placed into a drilled hole filled with injection mortar and is anchored via the bond between steel element, injection mortar and concrete. The steel element is intended to be used with embedment depth from 4 diameters to 20 diameters.

The illustration and the description of the product are given in Annexes A.

2 Specification of the intended use

The performances given in Section 3 are only valid if the anchor is used in compliance with the specifications and conditions given in Annexes B.

The provisions made in this European Technical Assessment are based on an assumed working life of the anchor of 50 years. The indications given on the working life cannot be interpreted as a guarantee given by the producer, but are to be regarded only as a means for choosing the right products in relation to the expected economically reasonable working life of the works.

3 Performance of the product

3.1 Mechanical resistance and stability (BWR 1)

Essential characteristic	Performance
Characteristic resistance under tension loads for threaded rod Acc. TR029	See Annex C1
Characteristic resistance under tension loads for rebars Acc. TR029	See Annex C2
Characteristic resistance under shear loads for threaded rods Acc. TR029	See Annex C3
Characteristic resistance under shear loads for rebars Acc. TR029	See Annex C4
Characteristic resistance under tension loads for threaded rods Acc. CEN/TS	See Annex C5
Characteristic resistance under tension loads for rebars Acc. CEN/TS	See Annex C6
Characteristic resistance under shear loads for threaded rods Acc. CEN/TS	See Annex C7
Characteristic resistance under shear loads for rebars Acc. CEN/TS	See Annex C8
Displacement for threaded rods and rebars	See Annex C9

3.2 Safety in case of fire (BWR 2)

Essential characteristic	Performance	
Reaction to fire	Anchorages satisfy requirements for Class A1	
Resistance to fire	No performance determined (NPD)	

3.3 Hygiene, health and the environment (BWR 3)

Regarding dangerous substances contained in this European Technical Assessment, there may be requirements applicable to the products falling within its scope (e.g. transposed European legislation and national laws, regulations and administrative provisions). In order to meet the provisions of the Construction Products Directive, these requirements need also to be complied with, when and where they apply.

3.4 Safety in use (BWR 4)

For Basic requirement Safety in use the same criteria are valid as for Basic Requirement Mechanical resistance and stability.

3.5 Protection against noise (BWR 5)

Not relevant.

3.6 Energy economy and heat retention (BWR 6)

Not relevant.

3.7 General aspects relating to fitness for use

Durability and Serviceability are only ensured if the specifications of intended use according to Annex B1 are kept.

4 Assessment and verification of constancy of performance (AVCP)

According to the Decision 96/582/EC of the European Commission¹, as amended, the system of assessment and verification of constancy of performance (see Annex V to Regulation (EU) No 305/2011) given in the following table apply.

Product	Intended use	Level or class	System
Metal anchors for use in concrete	For fixing and/or supporting to concrete, structural elements (which contributes to the stability of the works) or heavy units	_	1

5 Technical details necessary for the implementation of the AVCP system

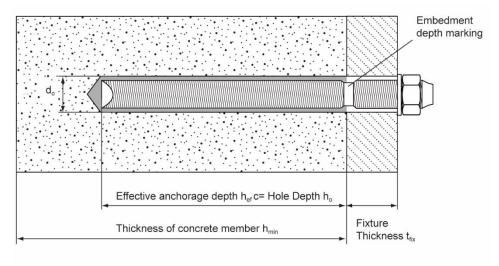
Technical details necessary for the implementation of the Assessment and verification of constancy of performance (AVCP) system are laid down in the control plan deposited at Centre Scientifique et Technique du Bâtiment.

The manufacturer shall, on the basis of a contract, involve a notified body approved in the field of anchors for issuing the certificate of conformity CE based on the control plan.

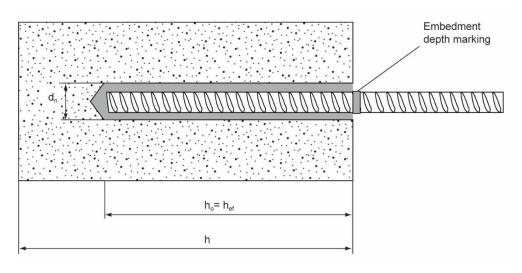
The original French version is signed by

Charles Baloche Technical Director

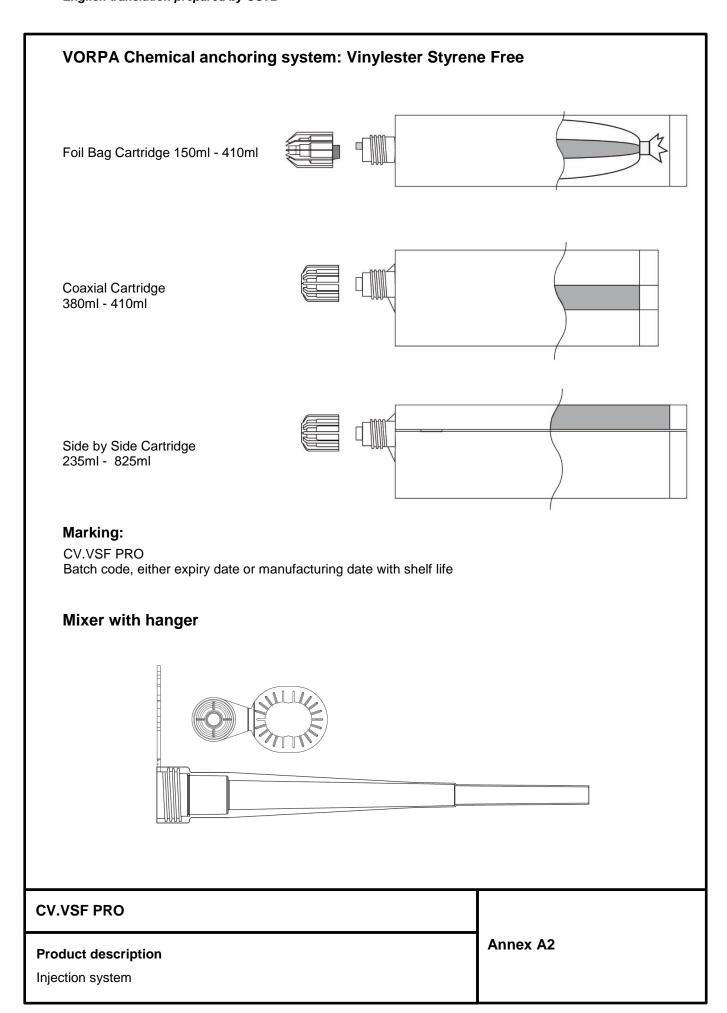
Threaded rod M8, M10, M12, M16, M20, M24



Reinforcing bar Ø8, Ø10, Ø12, Ø14, Ø16, Ø20, Ø25 acc. to Annex 4

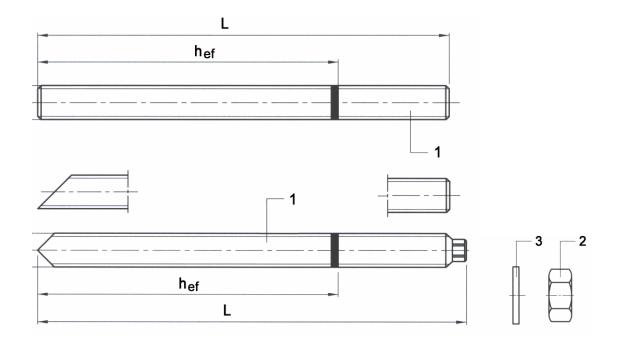


CV.VSF PRO	
Product description Installation condition	Annex A1



Anchor rod and rebar:

Threaded Steel Stud, Nut and Washer Sizes M8, M10, M12, M16, M20, M24.



Commercial standard rod with:

- Materials, dimensions and mechanical properties (Table 1a)
- Inspection certificate 3.1 acc. to EN 10204:2004
- Marking of embedment depth

Rebar

Diameter Ø 8mm, Ø 10mm, Ø 12mm, Ø 14mm, Ø 16mm, Ø 20mm, Ø 25mm



CV.VSF PRO	
Product description	Annex A3
Threaded rods and rebars	

Table A1: Materials

Designation	Material		
Threaded rods made of zinc coated steel			
Threaded rod M8 – M24	Strength class 5.8, 8.8, 10.9 EN ISO 898-1, Steel galvanized ≥ 5µm EN ISO 4042, Hot dipped galvanized ≥ 45 µm EN ISO 10684		
Washer ISO 7089	Steel galvanized EN ISO 4042; hot dipped galvanized EN ISO 10684		
Nut EN ISO 4032	Strength class 8 EN ISO 898-2 Steel galvanized ≥ 5 µm EN ISO 4042 Hot dipped galvanized ≥ 45 µm EN ISO 10684		
Threaded rods made of s	tainless steel		
Threaded rod M8 – M24	For ≤ M24: strength class 70 EN ISO 3506-1; Stainless steel 1.4401; 1.4404; 1.4578; 1.4571; 1.4439; 1.4362 EN 10088		
Washer ISO 7089	Stainless steel 1.4401; 1.4404; 1.4578; 1.4571; 1.4439; 1.4362 EN 10088		
Nut EN ISO 4032	Strength class 70 EN ISO 3506-2 Stainless steel 1.4401; 1.4404; 1.4578; 1.4571; 1.4439; 1.4362 EN 10088		
Threaded rods made of h	igh corrosion resistant steel		
Threaded rod M8 – M24	For \leq M20: $R_m = 800 \text{ N/mm}^2$; $R_{p0,2} = 640 \text{N/mm}^2$, For $>$ M20: $R_m = 700 \text{ N/mm}^2$; $R_{p0,2} = 400 \text{N/mm}^2$, High corrosion resistant steel 1.4529, 1.4565 EN 10088		
Washer ISO 7089	High corrosion resistant steel 1.4529, 1.4565 EN 10088		
Nut EN ISO 4032	Strength class 70 EN ISO 3506-2 High corrosion resistant steel 1.4529, 1.4565 EN 10088		

Table A2: Properties of reinforcement bars (rebars)

Product form		Bars and de-coiled rods	
Class		В	С
Characteristic yield streng	th f _{yk} or f _{0,2k} (MPa)	400 to 600	
Minimum value of k = (ft/ft	·y)k	≥ 1,08	≥ 1,15 < 1,35
Characteristic strain at ma	ıximum force, ε _{uk} (%)	≥ 5,0	≥ 7,5
Bendability		Bend / Rebend test	
Maximum deviation from nominal mass ≤ 8 (individual bar) (%) > 8		± 6	,
Bond: Minimum relative rib area, f _{R,min} (determination according to EN 15630) Nominal bar size (mm) 8 to 12 > 12		0,0 0,0	

Height of the rebar rib h_{rib}:

The height of the rebar rib h_{rib} shall fulfil the following requirement: 0,05 * d \leq h_{rib} \leq 0,07 * d with: d = nominal diameter of the rebar

CV.VSF PRO	
Product description Threaded rods and rebars	Annex A4

Specifications of intended use

Anchorages subject to:

· Static and quasi-static loads

Base materials:

- · Cracked concrete and non-cracked concrete
- Reinforced or unreinforced normal weight concrete of strength classes C 20/25 at least to C50/60 at most according to ENV 206: 2000-12.

Temperature Range:

- Ta: 40°C to +40°C (max. short term temperature +40°C and max. long term temperature +24°C)
- Tb: 40°C to +80°C (max. short term temperature +80°C and max. long term temperature +50°C)

Use conditions (Environmental conditions):

- Structures subject to dry internal conditions (zinc coated steel, stainless steel, high corrosion resistance steel).
- Structures subject to permanently damp internal condition, if no particular aggressive conditions exist (stainless steel, high corrosion resistance steel).
- Structures subject to permanently damp internal condition, with particular aggressive conditions (high corrosion resistance steel).
- Structures subject to external atmospheric exposure including industrial and marine environment if no particular aggressive conditions exist (stainless steel, high corrosion resistance steel).

Note: Particular aggressive conditions are e.g. permanent, alternating immersion in seawater or the splash zone of seawater, chloride atmosphere of indoor swimming pools or atmosphere with extreme chemical pollution (e.g. in desulphurization plants or road tunnels where de-icing materials are used).

Design:

- The anchorages are designed in accordance with the EOTA Technical Report TR 029 "Design of bonded anchors" and CEN/TS 1992-4-5" Design of fastenings for use in concrete" under the responsibility of an engineer experienced in anchorages and concrete work
- Verifiable calculation notes and drawings are prepared taking account of the loads to be anchored. The position of the anchor is indicated on the design drawings.

Installation:

- Dry or wet concrete (category 1).
- Hole drilling by rotary drill mode.
- Overhead installation is not permitted
- Installation in cracked concrete for threaded rods sizes M12 and M16 only
- Anchor installation carried out by appropriately qualified personnel and under the supervision of the person responsible for technical matters of the site.

CV.VSF PRO	
Intended Use	Annex B1
Specifications	

Table B1: Bore hole cleaning method with Steel brush

Threaded rod and rebar	Size	Nominal drill bit diameter d _o (mm)	Steel Brush	Cleaning methods	
		8		Manual cleaning (MAC)	Compressed air cleaning (CAC)
	M8	10	12mm	Yes h _{ef} ≤ 80 mm	
Studs	M10	12	14mm	Yes h _{ef} ≤ 100mm	
	M12	14	16mm	Yes h _{ef} ≤ 120mm	Yes
	M16	18	20mm	Yes h _{ef} ≤ 160mm	
	M20	24	26mm	Yes h _{ef} ≤ 200mm	
	M24	28	30mm	Yes h _{ef} ≤ 240mm	
	Ø8	12	14mm	Yes h _{ef} ≤ 80 mm	
 	Ø10	14	16mm	Yes h _{ef} ≤ 100mm	
Rebar	Ø12	16	18mm	Yes h _{ef} ≤ 120mm	
	Ø14	18	20mm	Yes h _{ef} ≤ 140mm	Yes
19191111111111111111111	Ø16	20	22mm	Yes h _{ef} ≤ 160mm	
	Ø20	25	28mm	Yes h _{ef} ≤ 200mm	
	Ø25	32	34mm	Yes h _{ef} ≤ 240mm	

Manual Cleaning (MAC):VORPA hand pump recommended for blowing out bore holes with diameters d₀ ≤ 24 mm and bore holes depth h₀ ≤10d



Compressed air cleaning (CAC): Recommended air nozzle with an opening of minimum 3,5mm in diameter.



CV.VSF PRO	
Intended Use	Annex B2
Cleaning brush Applicator guns	

Table B2a: Installation parameters: drilling, hole cleaning and installation

Instructions for use			
Bore hole drilling			
	Drill hole in the substrate to the required embedment depth using the appropriately sized carbide drill bit.		
Bore hole cleaning Just before	re setting an anchor, the bore hole must be free of dust and debris.		
a) Manual air cleaning (MAC) f	for all bore hole diameters d₀ ≤ 24mm and bore hole depth h₀ ≤ 10d		
X 4	The VORPA manual pump shall be used for blowing out bore holes up to diameters $d_0 \le 24$ mm and embedment depths up to $h_{ef} \le 10$ d. Blow out at least 4 times from the back of the bore hole, using an extension if needed.		
X 4	Brush 4 times with the specified brush size (see Table B1) by inserting the VORPA steel brush to the back of the hole (if needed with an extension) in a twisting motion and removing it.		
X 4	Blow out again with manual pump at least 4 times.		
b) Compressed air cleaning (C	CAC) for all bore hole diameters do and all bore hole depths		
6 Bar X 2	Blow 2 times from the back of the hole (if needed with a nozzle extension) over the whole length with oil-free compressed air (min. 6 bar at 6 m³/h).		
X 2	Brush 2 times with the specified brush size (see Table B1) by inserting the VORPA steel brush to the back of the hole (if needed with an extension) in a twisting motion and removing it.		
6 Ber X 2	Blow out again with compressed air at least 2 times.		

CV.VSF PRO	
Intended Use Manufacturer Published Installation Instructions	Annex B3

Table B2b: Installation parameters: drilling, hole cleaning and installation

Instructions for use	
	Remove the threaded cap from the cartridge.
	Tightly attach the mixing nozzle. Do not modify the mixer in any way. Made sure the mixing element is inside the mixer. Use only the supplied mixer.
	Insert the cartridge into the VORPA dispenser gun.
<u>*</u>	Discard the initial trigger pulls of adhesive. Depending on the size of the cartridge, an initial amount of adhesive mix must be discarded. Discard quantities are - 5cm for between 150ml, 300ml & 400ml Foil Pack - 10cm for all other cartridges
	Inject the adhesive starting at the back of the hole, slowly withdrawing the mixer with each trigger pull. Fill holes approximately 2/3 full, to ensure that the annular gap between the anchor and the concrete is completely filled with adhesive along the embedment depth.
h _{ef}	Before use, verify that the threaded rod is dry and free of contaminants. Install the threaded rod to the required embedment depth during the open gel time t_{gel} has elapsed. The working time t_{gel} is given in Table B3.
t _{cure} T _{inst}	The anchor can be loaded after the required curing time t_{cure} (seeTable B3) The applied torque shall not exceed the values T_{max} given in Table B4.

CV.VSF PRO	
Intended Use Manufacturer Published Installation Instructions	Annex B3

Table B3: Minimum curing time

	Mini		aterial temperature C°	Gel time (working time) t _{ge} l In dry/wet concrete	Cure time
0°	≤	T _{base material}	< 5°C	25 min	90 min
5°C	≤	T _{base material}	< 10°C	17 min	70 min
10°C	≤	T _{base material}	< 20°C	12 min	65 min
20°C	≤	T _{base material}	< 30°C	6 min	60 min
30°C	<u>≤</u>	T _{base material}	≤ 40°C	3 min	45 min

The temperature of the bond material must be ≥ 20 °C

CV.VSF PRO	
Intended Use Gelling and curing times	Annex B4

Anchor size	-		M8	M10	M12	M16	M20	M24
Diameter of anchor rod	d	[mm]	8	10	12	16	20	24
Range of anchorage depth hef	min	[mm]	60	60	70	80	90	100
and bore hole depth ho	max	[mm]	160	200	240	320	400	480
Nominal anchorage depth	h _{ef}	[mm]	80	90	110	125	170	210
Nominal diameter of drill bit	d₀	[mm]	10	12	14	18	24	28
Diameter of clearance hole in the fixture	df	[mm]	9	12	14	18	22	26
Maximum torque moment	T_{max}	[Nm]	10	20	30	60	90	140
Minimum thickness of concrete member	h _{min}	[mm]	_	+ 30m 100mr		h _{ef} + 2d _o		0
Minimum spacing	Smin	[mm]	40	50	60	80	100	120
Minimum edge distance	C_{min}	[mm]	40	50	60	80	100	120

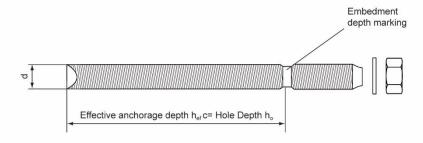
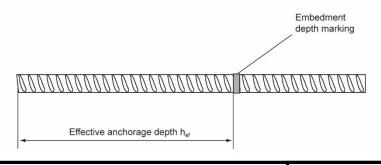


Table B5 - Installation details for rebars

Rebar Diameter				Ø10	Ø12	Ø14	Ø16	Ø20	Ø25
Diameter of element	D	[mm]	8	10	12	14	16	20	25
Range of anchorage depth hef	min	[mm]	60	60	70	75	80	90	100
and bore hole depth h₀	max	[mm]	160	200	240	280	320	400	500
Nominal diameter of drill bit	d₀	[mm]	12	14	16	18	20	25	32
Minimum thickness of concrete member	h _{min}	[mm]		_{ef} + 30m ≥ 100mr		h _{ef} + 2d _o			
Minimum spacing	Smin	[mm]	40	50	60	70	80	100	125
Minimum edge distance	Cmin	[mm]	40	50	60	70	80	100	125



CV.VSF PRO	
Intended Use Installation parameters	Annex B5

CV.VSF PRO with threaded roo	ds		M8	M10	M12	M16	M20	M24		
Steel failure		<u>'</u>					•			
Characteristic resistance, class 5.8	$N_{\text{Rk},s}$	[kN]	18	29	42	79	123	177		
Characteristic resistance, class 8.8	$N_{\text{Rk},\text{s}}$	[kN]	29	46	67	126	196	282		
Partial safety factor	γMs,N ¹⁾	[-]			1	,5				
Characteristic resistance, class 10.9	$N_{Rk,s}$	[kN]	36	58	84	157	245	353		
Partial safety factor	γMs,N ¹⁾	[-]	[-] 1.4							
Characteristic resistance, A4-70	$N_{Rk,s}$	[kN]	26	41	59	110	172	247		
Partial safety factor	γ _{Ms,N} 1)	[-]			1,	87				
Characteristic resistance, HCR	$N_{Rk,s}$	[kN]	29	46	67	126	196	247		
Partial safety factor	γMs,N ¹⁾	[-]			1,5			2,1		
Combined Pull-out and Concrete co	one failure									
Diameter of threaded rod	d	[mm]	8	10	12	16	20	24		
Characteristic bond resistance in non-	cracked co	ncrete C20/	25							
Temperature range I ²⁾ : 40°C/24°C	τ_{Rk}	[N/mm²]	10.0	9.5	9.0	8.0	7.5	7.0		
Temperature range II ²⁾ : 80°C/50°C	τ _{Rk}	[N/mm²]	9.0	8.0	7.5	7.0	6.5	6.0		
		C30/37	1,12							
Increasing factor for TRK,p in non-cracked concrete	ψc -	C40/50			1,	23				
III HOH-Clacked Concrete		C50/60	1,30							
Characteristic bond resistance in crac	ked concre	te C20/25								
Temperature range I ²⁾ : 40°C/24°C	$ au_{Rk}$	[N/mm²]	_5)	_5)	3.5	3.5	_5)	_ 5)		
Temperature range II ²⁾ : 80°C/50°C	τ_{Rk}	[N/mm²]	_5)	_5)	3.0	3.0	_5)	_5)		
	_	C30/37			1,	04				
Increasing factor for TRK,p in cracked concrete	ψc	C40/50			1,	07				
in ordered concrete		C50/60			1,	09				
Splitting failure ²⁾										
	h / h	n _{ef} ³⁾ ≥ 2,0	1,0	h _{ef}	h/h _{ef}					
Edge distance c _{cr,sp} [mm] for	2,0 > h / h _{ef} ³⁾ > 1,3		4,6 h _{ef}	- 1,8 h	1,3					
$h / h_{ef}^{3)} \le 1,3$			2,26 h _{ef} c _{cr,s}					C _{cr,sp}		
Spacing	Scr,sp	[mm]			2 0	cr,sp				
Partial safety factor $\gamma_{Mp} = \gamma_{Mo}$	$= \gamma_{\rm Msp}^{-1}$	[-]	1,5 ⁴⁾	1,5 ⁴⁾	1,5 ⁴⁾	1,5 ⁴⁾	1,5 ⁴⁾	1,5 4		

- 1) In absence of national regulations
- 2) Explanations, see Annex B1
- 3) h. concrete member thickness, h_{ef} effective anchorage depth
- 4) The partial safety factor $\gamma_2 = 1.0$ is included
- 5) Not qualified in cracked concrete

CV.VSF PRO	
Design according to TR 029	Annex C1
Characteristic resistance under tension loads for threaded rods	

CV.VSF PRO with rebar			Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25	
Steel failure rebar										
Characteristic resistance for rebar BSt 500 S acc. to DIN 488 $^{1)}$	$N_{Rk,s}$	[kN]	28	43	62	85	111	173	270	
Partial safety factor for rebar BSt 500 S acc. to DIN 488 ²⁾	γMs,N ³⁾	[-]				1,4				
Combined Pull-out and Concrete cone failure										
Diameter of rebar	d	[mm]	8	10	12	14	16	20	25	
Characteristic bond resistance in non-cracked concrete C2										
Temperature range I ⁴⁾ : 40°C/24°C	$ au_{Rk}$	[N/mm²]	7,0	7,5	7,0	7,0	6,5	6,5	6,0	
Temperature range II ⁴⁾ : 80°C/50°C	$ au_{Rk}$	[N/mm²]	6.5	6.5	6,0	6,0	6,0	5,5	5,5	
		C30/37				1,12		•	•	
Increasing factor for T _{Rk,p}	ψc	C40/50	1,23							
in non-cracked concrete	•	C50/60	1,30							
Splitting failure										
_	h /	′ h _{ef} ⁵⁾ ≥ 2,0	1,	0 h _{ef}	h/i					
Edge distance c _{cr,sp} [mm] for	2,0 > h /	h _{ef} ⁵⁾ > 1,3	4,6 h	_{ef} - 1,8 h	1,	3 -				
	h /	h _{ef} ⁵⁾ ≤ 1,3	2,2	26 h _{ef}			1,0·h _{ef}	2,26·h _{ef}	C _{cr,sp}	
Spacing	S _{cr,sp}	[mm]				2 C _{cr,sp}				
Partial safety factor $\gamma_{Mp} = \gamma_{Mo}$	$s = \gamma_{Msp}^{(3)}$	[-]	1,8 ⁶⁾	1,8 ⁶⁾	1,8 ⁶⁾	1,8 ⁶⁾	1,8 ⁶⁾	1,8 ⁶⁾	1,8 ⁶⁾	

- 1) The characteristic tension resistance N_{Rk,s} for rebars that do not fulfill the requirements acc. DIN 488 shall be calculated acc. Technical Report TR029, Equation (5.1).
- The partial safety factor $\gamma_{Ms,N}$ for rebars that do not fulfill the requirements acc. DIN 488 shall be calculated acc. Technical Report TR029, Equation (3.3a).
- 3) In absence of national regulations
- 4) Explanation see Annex B1
- $^{5)}\,\,$ h concrete member thickness, $h_{\text{ef}}\,$ effective anchorage depth
- $^{6)}$ $\;$ The partial safety factor γ_2 = 1,2 is included.

CV.VSF PRO	
Design according to TR 029	Annex C2
Characteristic resistance under tension loads for rebars	

CV.VSF PRO with threaded rods			M 8	M 10	M 12	M 16	M 20	M 24
Steel failure without lever arm								
Characteristic resistance, class 5.8	$V_{\text{Rk},\text{s}}$	[kN]	9	15	21	39	61	88
Characteristic resistance, class 8.8	$V_{Rk,s}$	[kN]	15	23	34	63	98	141
Characteristic resistance, class 10.9	$V_{Rk,s}$	[kN]	18	29	42	79	123	156
Characteristic resistance, A4-70	$V_{Rk,s}$	[kN]	13	20	30	55.0	86	124
Characteristic resistance, HCR	$V_{\text{Rk},s}$	[kN]	15	23	34	62.8	98	124
Steel failure with lever arm								
Characteristic resistance, class 5.8	$M^0_{Rk,s}$	[Nm]	19	37	66	167	326	561
Characteristic resistance, class 8.8	M^0 Rk,s	[Nm]	30.0	60	105	266	519	898
Characteristic resistance, class 10.9	M^0 Rk,s	[Nm]	38	75	131	333	649	893
Characteristic resistance, A4-70	M^0 Rk,s	[Nm]	26	53	92	233	454	625
Characteristic resistance, HCR	M^0 Rk,s	[Nm]	30	60	105	266	519	786
Partial safety factor steel failure								
grade 5.8 or 8.8	$\gamma_{Ms,V}^{1)}$	[-]			1,:	25		
grade 10.9	γMs,V ¹⁾	[-]			1,	50		
A4-70	$\gamma_{Ms,V}$ 1	[-]			1,	56		
HCR	$\gamma_{Ms,V}$ 1)	[-]			1,25			1,75
Concrete pryout failure								
Factor in equation (5.7) of Technical Report TR029 for the design of bonded anchors	k	[-]	2,0					
Partial safety factor	γ _{Mcp} 1)	[-]	1,5 ²⁾					
Concrete edge failure ³⁾								
Partial safety factor	γMc ¹⁾	[-]			1,5	5 ²⁾		

¹⁾ In absence of national regulations

CV.VSF PRO	
Design according to TR 029	Annex C3
Characteristic resistance under shear loads for threaded rods	

²⁾ The partial safety factor γ_2 = 1.0 is included

³⁾ Concrete edge failure see chapter 5.2.3.4 of Technical Report TR029

CV.VSF PRO with rebar			Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25
Steel failure without lever arm									
Characteristic shear resistance for rebar BSt 500 S acc. to DIN 4881)	$V_{Rk,s}$	[kN]	14	22	31	42	55	86	135
Partial safety factor for rebar BSt 500 S acc. to DIN 488 ²⁾	γMs,V ³⁾	[-]				1,5			
Steel failure with lever arm									
Characteristic shear resistance for rebar BSt 500 S acc. to DIN 488 ⁴⁾	$M^0_{Rk,s}$	[Nm]	33	65	112	178	265	518	1012
Partial safety factor for rebar BSt 500 S acc. to DIN 488 3)	γMs,V ³⁾	[-]	1,5						
Concrete pryout failure									
Factor in equation (5.7) of Technical Report TR029 for the design of bonded anchors	k	[-]	2,0						
Partial safety factor	γ _{Mcp} ³⁾	[-]	1,5 ⁵⁾						
Concrete edge failure ⁶⁾									
Partial safety factor	γMc ³⁾	[-]				1,5 ⁵⁾			

- ¹⁾ The characteristic shear resistance $V_{Rk,s}$ for rebars that do not fulfill the requirements acc. DIN 488 shall be calculated acc. Technical Report TR029, Equation (5.6).
- The partial safety factor $\gamma_{Ms,N}$ for rebars that do not fulfill the requirements acc. DIN 488 shall be calculated acc. Technical Report TR029, Equation 3.3b or 3.3c.
- 3) In absence of national regulations
- The characteristic bending resistance $M^0_{Rk,s}$ for rebars that do not fulfill the requirements acc. DIN 488 shall be calculated acc. Technical Report TR029, Equation (5.6b).
- ⁵⁾ The partial safety factor $\gamma_2 = 1.0$ is included.
- ⁶⁾ Concrete edge failure see chapter 5.2.3.4 of Technical Report TR029

CV.VSF PRO	
Design according to TR 029 Characteristic resistance under shear loads for rebars	Annex C4

CV.VSF PRO with threaded rods	M 8	M 10	M 12	M 16	M 20	M 24			
Steel failure									
Characteristic resistance, class 5.8	$N_{Rk,s}$	[kN]	18	29	42	79	123	177	
Characteristic resistance, class 8.8	$N_{Rk,s}$	[kN]	29	46	67	126	196	282	
Partial safety factor	γMs,N ¹⁾	[-]			1.	50			
Characteristic resistance, class 10.9	$N_{Rk,s}$		36	58	84	157	245	353	
Partial safety factor	γ _{Ms,N} 1)				1.4	40			
Characteristic resistance "A4 70"	$N_{Rk,s}$	[kN]	26	41	59	110	172	247	
Partial safety factor	γMs,N ¹⁾	[-]			1.8	87			
Characteristic resistance "HCR"	$N_{Rk,s}$	[kN]	29	46	67	126	196	247	
Partial safety factor	γMs,N ¹⁾	[-]			1.5			2.1	
Combined Pull-out and Concrete cone									
Diameter of threaded rod	d	[mm]	8	10	12	16	20	24	
Characteristic bond resistance in non-crac	ked concrete C	20/25							
Temperature range I ²⁾ : 40°C/24°C	$ au_{Rk,uncr}$	[N/mm²]	10.0	9.5	9.0	8.0	7.5	7.0	
Temperature range II ²⁾ : 80°C/50°C	$ au_{Rk,uncr}$	[N/mm²]	9.0	8.0	7.5	7.0	6.5	6.0	
		C30/37			1,	12			
Increasing factor for $\tau_{Rk,p}$	Ψc	C40/50	1,23						
in non-cracked concrete		C50/60	1,30						
Characteristic bond resistance in cracked	concrete C20/2	5							
Temperature range I ²⁾ : 40°C/24°C	$ au_{Rk,cr}$	[N/mm²]	_5)	_5)	3.5	3.5	_ 5)	_ 5)	
Temperature range II ²⁾ : 80°C/50°C	$ au_{Rk,cr}$	[N/mm²]	_5)	_5)	3.0	3.0	_5)	_ 5)	
la ana asia a fa atau fa a =		C30/37			1,0				
Increasing factor for τ_{Rk} in cracked concrete	Ψc	C40/50	1,07						
		C50/60	1,09						
Factor according to CEN/TS 1992-4-5	k8 non cracked cond		10.1						
Section 6.2.2	K8 cracked concrete	[-]			7.	.2			
Concrete cone failure	L _c	r 1			10	. 1			
Factor according to CEN/TS 1992-4-5 Section 6.2.3	k _{ucr} k _{cr}	[- <u>]</u> [-]			10				
Edge distance	C _{cr,N}	[-]	7.2 1,5 h _{ef}						
Axial distance	S _{cr,N}	[-]			3,0				
Splitting failure	Oci,N	LJ			0,0	1161			
Ophicing landle				I	n/h _{ef} †				
	h / h _{ef} ³⁾ ≥ 2,0		1,0 l	Λef	2,0				
Edge distance c _{cr,sp} [mm] for	$2.0 > h / h_{ef}^{(3)} > 1.3$ $h / h_{ef}^{(3)} \le 1.3$		4,6 h _{ef} -	1,8 h	1,3				
			2,26	h _{ef}	1	1,0·h _{ef}	2,26·h _{ef}	C _{cr,sl}	
Spacing	$S_{cr,sp}$	[mm]			2.0	cr,sp			

- 1) In absence of national regulations
- 2) Explanations, see Annex B1
- $^{\rm 3)}$ $\,$ h concrete member thickness, $h_{\rm ef}$ $\,$ effective anchorage depth
- The partial safety factor $\gamma_2 = 1.0$ is included
- 5) Not qualified in cracked concrete

CV.VSF PRO Design according to CEN/TS 1992-4 Characteristic resistance under tension loads for threaded rods Annex C5

CV.VSF PRO with rebar			Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25
Steel failure rebar									
Characteristic resistance for rebar BSt 500 S acc. to DIN 488 1)	$N_{\text{Rk,s}}$	[kN]	28	43	62	85	111	173	270
Partial safety factor for rebar BSt 500 S acc. to DIN 488 $^{2)}$	'Ms,N ³⁾	[-]				1,4			
Combined Pull-out and Concrete cone	failure								
Diameter of rebar	d	[mm]	8	10	12	14	16	20	25
Characteristic bond resistance in non-crac	ked co	ncrete C20/	/25						
Temperature range I ⁴⁾ : 40°C/24°C	τ_{Rk}	[N/mm²]	7,0	7,5	7,0	7,0	6,5	6,5	6,0
Temperature range II ⁴⁾ : 80°C/50°C	τ _{Rk}	[N/mm²]	6.5	6.5	6,0	6,0	6,0	5,5	5,5
		C30/37				1,12			
Increasing factor for TRk,p in non-cracked concrete	ψc	C40/50				1,23			
III HOH-GIACKEU COHCIETE	-	C50/60				1,30			
Factor according to CEN/TS 1992-4-5 Section 6.2.2	cracked c	concrete [-]				10.1			
Concrete cone failure									
Factor according to CEN/TS 1992-4-5 Section 6.2.2	k u	icr [-]				10.1			
Splitting failure									
	h /	′ h _{ef} ⁵⁾ ≥ 2,0	1,	0 h _{ef}	h/t 				
Edge distance c _{cr,sp} [mm] for 2,	0 > h /	h _{ef} ⁵⁾ > 1,3	4,6 h	_{ef} - 1,8 h	1,3	3 -			
	h /	h _{ef} ⁵⁾ ≤ 1,3	2,26 h _{ef}			1	1,0·h _{ef}	2,26·h _{ef}	$\mathbf{c}_{cr,sp}$
Spacing	S _{cr,sp} [mm]								
Partial safety factor $\gamma_{Mp} = \gamma_{Mc} = \gamma$	/Msp ³⁾	[-]	1,8 ⁶⁾	1,8 ⁶⁾	1,8 ⁶⁾	1,8 ⁶⁾	1,8 ⁶⁾	1,8 ⁶⁾	1,8 ⁶⁾

- The characteristic tension resistance $N_{Rk,s}$ for rebars that do not fulfill the requirements acc. DIN 488 shall be calculated acc. Technical Report TR029, Equation (5.1).
- The partial safety factor γ_{Ms,N} for rebars that do not fulfill the requirements acc. DIN 488 shall be calculated acc. Technical Report TR029, Equation (3.3a).
- 3) In absence of national regulations
- 4) Explanation see Annex B1
- $^{5)}\,\,$ h concrete member thickness, $h_{\text{ef}}\,$ effective anchorage depth
- $^{6)}$ $\;$ The partial safety factor γ_2 = 1,2 is included.

CV.VSF PRO	
Design according to CEN/TS 1992-4	Annex C6
Characteristic resistance under tension loads for rebars	

CV.VSF PRO with threaded rods			M 8	M 10	M 12	M 16	M 20	M 24
Steel failure without lever arm								
Characteristic resistance, class 5.8	$V_{Rk,s}$	[kN]	9	15	21	39	61	88
Characteristic resistance, class 8.8	$V_{Rk,s}$	[kN]	15	23	34	63	98	141
Characteristic resistance, class 10.9	$V_{Rk,s}$	[kN]	18	29	42	79	123	156
Characteristic resistance, A4-70	$V_{Rk,s}$	[kN]	13	20	30	55.0	86	124
Characteristic resistance, HCR	$V_{Rk,s}$	[kN]	15	23	34	62.8	98	124
Steel failure with lever arm								
Characteristic resistance, class 5.8	$M^0_{Rk,s}$	[Nm]	19	37	66	167	326	561
Characteristic resistance, class 8.8	$M^0_{Rk,s}$	[Nm]	30.0	60	105	266	519	898
Characteristic resistance, class 10.9	M^0 Rk,s	[Nm]	38	75	131	333	649	893
Characteristic resistance, A4-70	M^0 Rk,s	[Nm]	26	53	92	233	454	625
Characteristic resistance, HCR	$M^0_{Rk,s}$	[Nm]	30	60	105	266	519	786
Partial safety factor steel failure								
grade 5.8 or 8.8	γMs,V ¹⁾	[-]			1,	25		
grade 10.9	γMs,V ¹⁾	[-]			1,	50		
A4-70	γMs,V ¹⁾	[-]			1,	56		
HCR	γMs,V ¹⁾	[-]	1,25 1,					1,75
Concrete pryout failure								
Factor according to CEN/TS 1992-4-5 Section 4.3.3	k ₃	[-]	2,0					
Partial safety factor	γMcp ¹⁾	[-]	1,5 ²⁾					
Concrete edge failure								
Partial safety factor	γMc ¹⁾	[-]			1,	5 ²⁾		

¹⁾ In absence of national regulations

CV.VSF PRO	
Design according to CEN/TS 1992-4	Annex C7
Characteristic resistance under shear loads for threaded rods	

²⁾ The partial safety factor $\dot{\gamma}_{2}$ = 1.0 is included

CV.VSF PRO with rebar			Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25
Steel failure without lever arm									
Characteristic shear resistance for rebar BSt 500 S acc. to DIN 4881)	$V_{Rk,s}$	[kN]	14	22	31	42	55	86	135
Partial safety factor for rebar BSt 500 S acc. to DIN 488 ²⁾	$\gamma_{Ms,V}^{3)}$	[-]	1,5						
Steel failure with lever arm									
Characteristic shear resistance for rebar BSt 500 S acc. to DIN 4881)	M^0 _{Rk,s}	[Nm]	33	65	112	178	265	518	1012
Partial safety factor for rebar BSt 500 S acc. to DIN 488 2)	$\gamma_{Ms,V}^{(3)}$	[-]	1,5						
Concrete pryout failure									
Factor according to CEN/TS 1992-4-5 Section 4.3.3	k ₃	[-]	2,0						
Partial safety factor	γMcp ³⁾	[-]	1,5 ⁵⁾						
Concrete edge failure									
Partial safety factor	γ Mc $^{3)}$	[-]		·		1,5 ⁵⁾	·		·

- The characteristic shear resistance $V_{Rk,s}$ for rebars that do not fulfill the requirements acc. DIN 488 shall be calculated acc. Technical Report TR029, Equation (5.6).
- The partial safety factor $\gamma_{\text{Ms,N}}$ for rebars that do not fulfill the requirements acc. DIN 488 shall be calculated acc. Technical Report TR029, Equation 3.3b or 3.3c.
- 3) In absence of national regulations
- ⁴⁾ The characteristic bending resistance M⁰_{Rk,s} for rebars that do not fulfill the requirements acc. DIN 488 shall be calculated acc. Technical Report TR029, Equation (5.6b).
- $^{5)}$ $\;$ The partial safety factor γ_2 = 1,0 is included.

CV.VSF PRO	
Design according to CEN/TS 1992-4	Annex C8
Characteristic resistance under shear loads for rebars	

Displacement under tension load 1)

CV.VSF PRO with threaded rods			M8	M10	M12	M16	M20	M24
Non cracked concrete temperature range I 7): 40°C / 24°C								
Displacement	δ_{N0}	[mm/(N/mm²)]	0,03	0,03	0,04	0,05	0,06	0,07
Displacement	δn∞	[mm/(N/mm²)]	0,07	0,09	0,10	0,13	0,17	0,20
Non cracked concrete temperature range II 7): 80°C / 50°C								
Displacement	δηο	$[mm/(N/mm^2)]$	0,04	0,04	0,05	0,07	0,08	0,10
Displacement	δn∞	[mm/(N/mm²)]	0,10	0,13	0,15	0,19	0,23	0,28
Cracked concrete temperature range I ⁷): 40°C / 24°C								
Displacement	δηο	$[mm/(N/mm^2)]$	-	-	0,12	0,09	-	-
Displacement	δn∞	[mm/(N/mm²)]	-	-	0,64	0,55	-	-
Cracked concrete temperature range II 7): 80°C / 50°C								
Displacement	δ_{N0}	[mm/(N/mm²)]	-	-	0,17	0,13	-	-
Displacement	δ _{N∞}	[mm/(N/mm²)]	-	-	0,90	0,78	-	-

CV.VSF PRO with rebar			Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25
Temperature range I ⁹⁾ : 40°C / 24°C									
Displacement	δηο	[mm/(N/mm²)]	0,03	0,03	0,04	0,04	0,05	0,06	0,07
Displacement	δ _{N∞}	[mm/(N/mm²)]	0,07	0,09	0,10	0,12	0,13	0,17	0,20
Temperature range II ⁹⁾ : 80°C / 50°C									
Displacement	δ_{N0}	$[mm/(N/mm^2)]$	0,04	0,04	0,05	0,06	0,07	0,08	0,10
Displacement	δ _{N∞}	[mm/(N/mm²)]	0,10	0,13	0,15	0,17	0,19	0,23	0,29

Calculation of displacement under service load: τ_{Sd} design value of bond stress Displacement under short term loading = $\delta_{N0} \cdot \tau_{Sd}/1,4$ Displacement under long term loading = $\delta_{N\infty} \cdot \tau_{Sd}/1,4$

Displacement under shear load 2)

CV.VSF PRO with threaded rods			M8	M10	M12	M16	M20	M24
Displacement	δ_{V0}	[mm/kN]	0,06	0,06	0,05	0,04	0,04	0,03
Displacement	δν∞	[mm/kN]	0,09	0,08	0,08	0,06	0,06	0,05

CV.VSF PRO with rebar			Ø8	Ø10	Ø12	Ø14	Ø16	Ø20	Ø25
Displacement	δνο	[mm/kN]	0,06	0,05	0,05	0,04	0,04	0,04	0,03
Displacement	$\delta_{\text{V}\infty}$	[mm/kN]	0,09	0,08	0,07	0,06	0,06	0,05	0,05

²⁾ Calculation of displacement under service load: V_{Sd} design value of shear load. Displacement under short term loading = $\delta_{N0} \cdot V_{Sd}/1,4$ Displacement under long term loading = $\delta_{V\infty} \cdot V_{Sd}/1,4$

CV.VSF PRO	
Design	Annex C9
Anchor displacements	